## Low-cycle Fatigue Modelling of Steel Beam-column Connections Based on Lumped Damage Mechanics

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## 1. Introduction

In Northridge earthquake 1994 and Hyogoken-Nanbu earthquake 1995, brittle fractures were observed at beam-to-column welded connections of steel moment-resisting frame buildings. Such damage significantly reduced capacity of beams to resist against earthquake loading, which in turn triggered damage in columns and collapse of buildings. This paper introduces a novel method to simulate beam fractures using Lumped Damage Mechanics (LDM), which can directly represent damage consequences, i.e. stiffness and strength reduction, by a single damage variable integrated in a material constitutive model.

## 2. Concept of lumped damage mechanics

In LDM, a frame element is assumed as the assemblage of an elastic beam and two inelastic hinges located at its ends as shown in Fig. 1. Plastic deformations ( $\phi_i^p$  and  $\phi_j^p$ ) and damage variables ( $d_i$  and  $d_j$ ) are introduced in the inelastic hinges.

The damage evolution law for simulating fracture is defined based on the Coffin-Manson law for low-cycle fatigue (LCF) where the number of cycles to fracture, N, in a uniaxial test with constant plastic strain amplitude is expressed as:

$$N = \left(\frac{\Delta \varepsilon_p}{\varepsilon_f}\right)^{\beta} \tag{1}$$

where  $\Delta \varepsilon_p$  is the plastic strain amplitude,  $\varepsilon_f$  and  $\beta$  are material parameters. In particular,  $\varepsilon_f$  indicates the plastic strain amplitude that produces fracture in one cycle and  $\beta$  represents sensitivity of plastic strain amplitude with the number of cycles to initiate fracture.



## 3. Numerical simulation of low-cycle fatigue

The lumped damage model requires the computation of conventional parameters in a frame element with plastic hinges (EI, AE,  $M_{y}$ , C) and three non-conventional ones for this kind of structures:  $\alpha$ ,  $\beta$ and  $\phi_{cr}$ . Fig. 2 shows the simulation results of low-cycle fatigue fractures of two beam-column connection specimens under constant loading amplitude. It is noted that the ST-1 and ST-2 specimens experienced significant strength degradation accompanying with the increasing of the number of loading cycle. The proposed lumped damage model achieved high accuracy for simulating LCF fracture of steel beam-column connections.



Fig. 2 Simulation results of low-cycle fatigue fracture